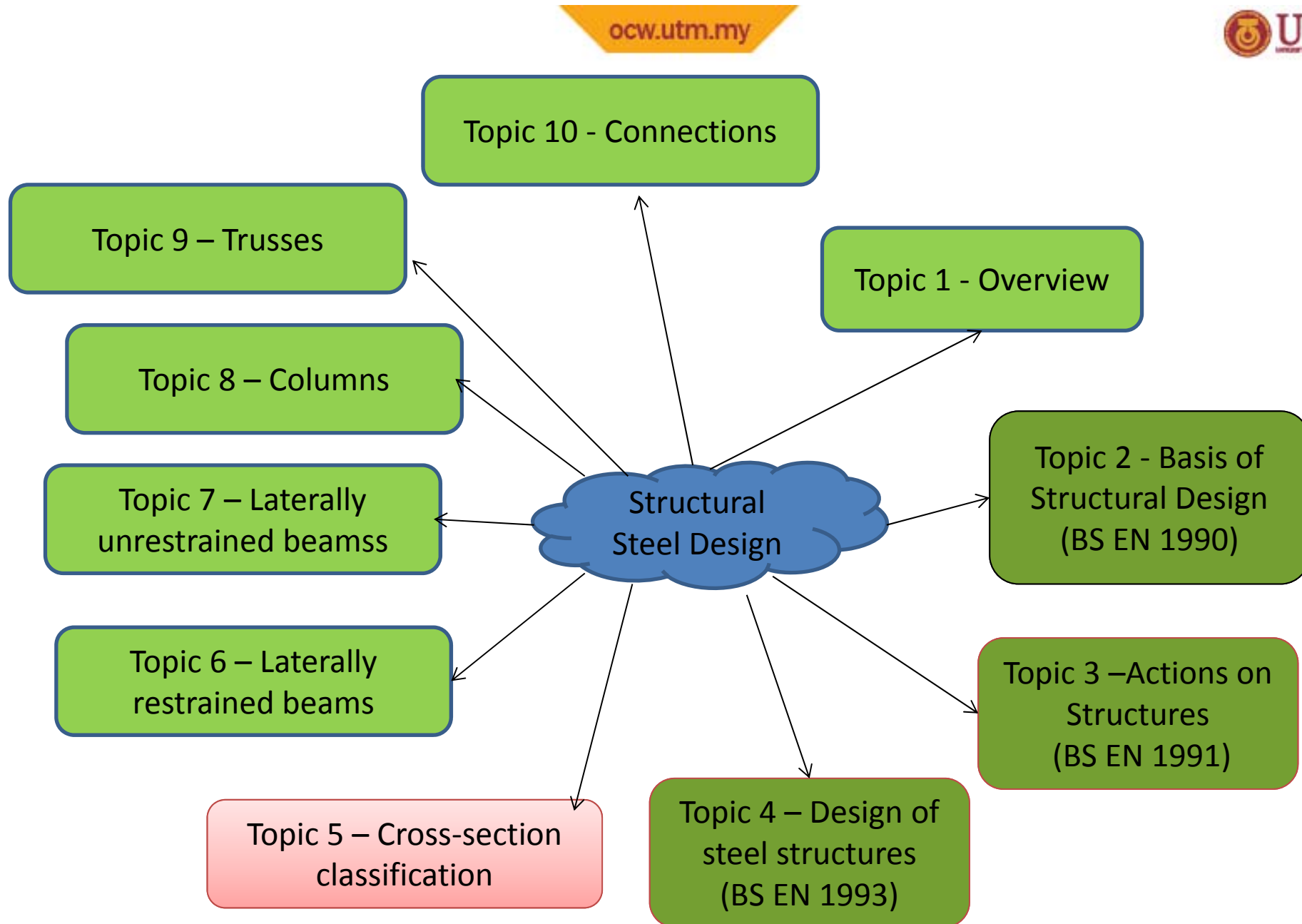


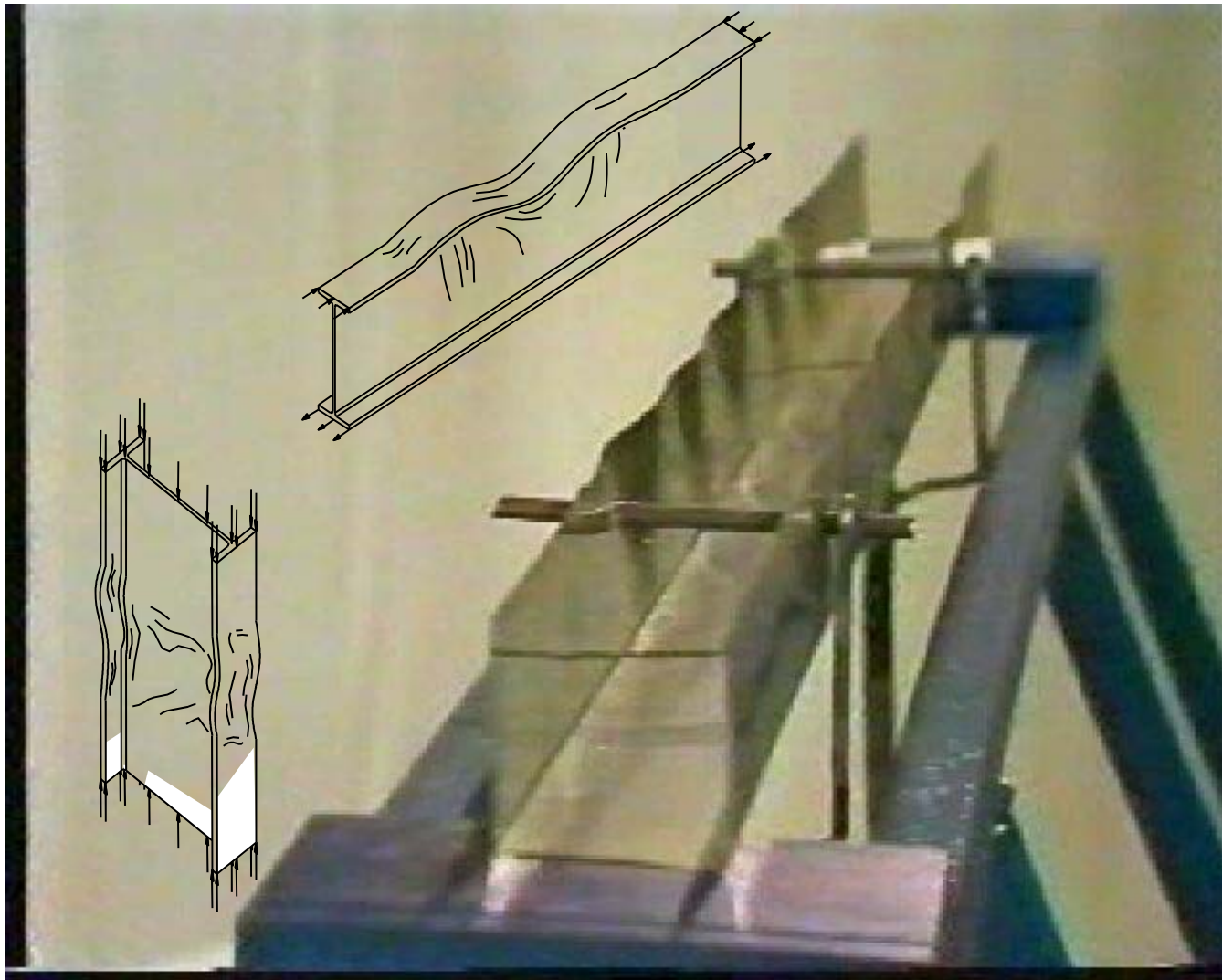
# Structural Steel and Timber Design SAB3233

## Topic 5 Cross-section classification

Prof Dr Shahrin Mohammad





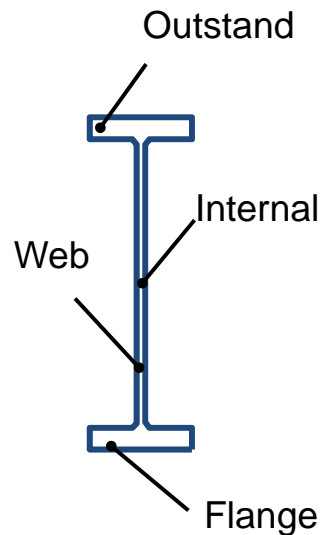


# Basis of section classification

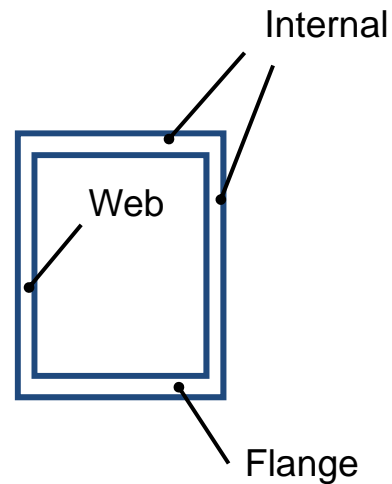
Rolled or welded sections may be considered an assembly of individual plate elements

Some are outstand  
- flanges of I beams  
- legs of angles and Tees

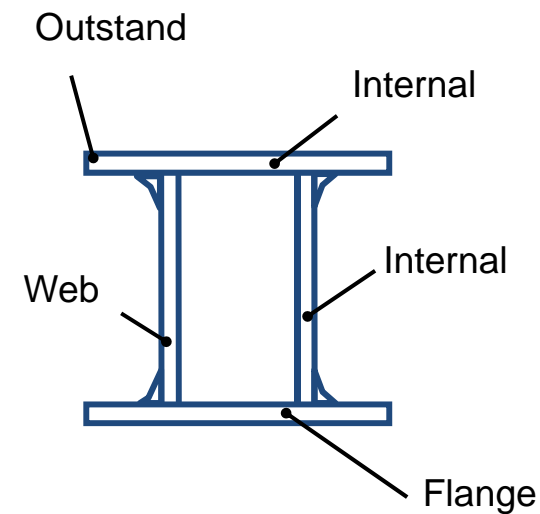
Some are internal  
- webs of open beams  
- flanges of boxes



Rolled I-section



Hollow section



Welded box section

## Basis of section classification

- As the plate elements are relatively thin, when loaded in compression they may buckle locally
- The tendency of any plate element within the cross section to buckle may limit the axial load carrying capacity, or the bending resistance of the section, by preventing the attainment of yield.
- Avoidance of premature failure arising from the effects of local buckling may be achieved by limiting the width-to-thickness ratio for individual elements within the cross section.

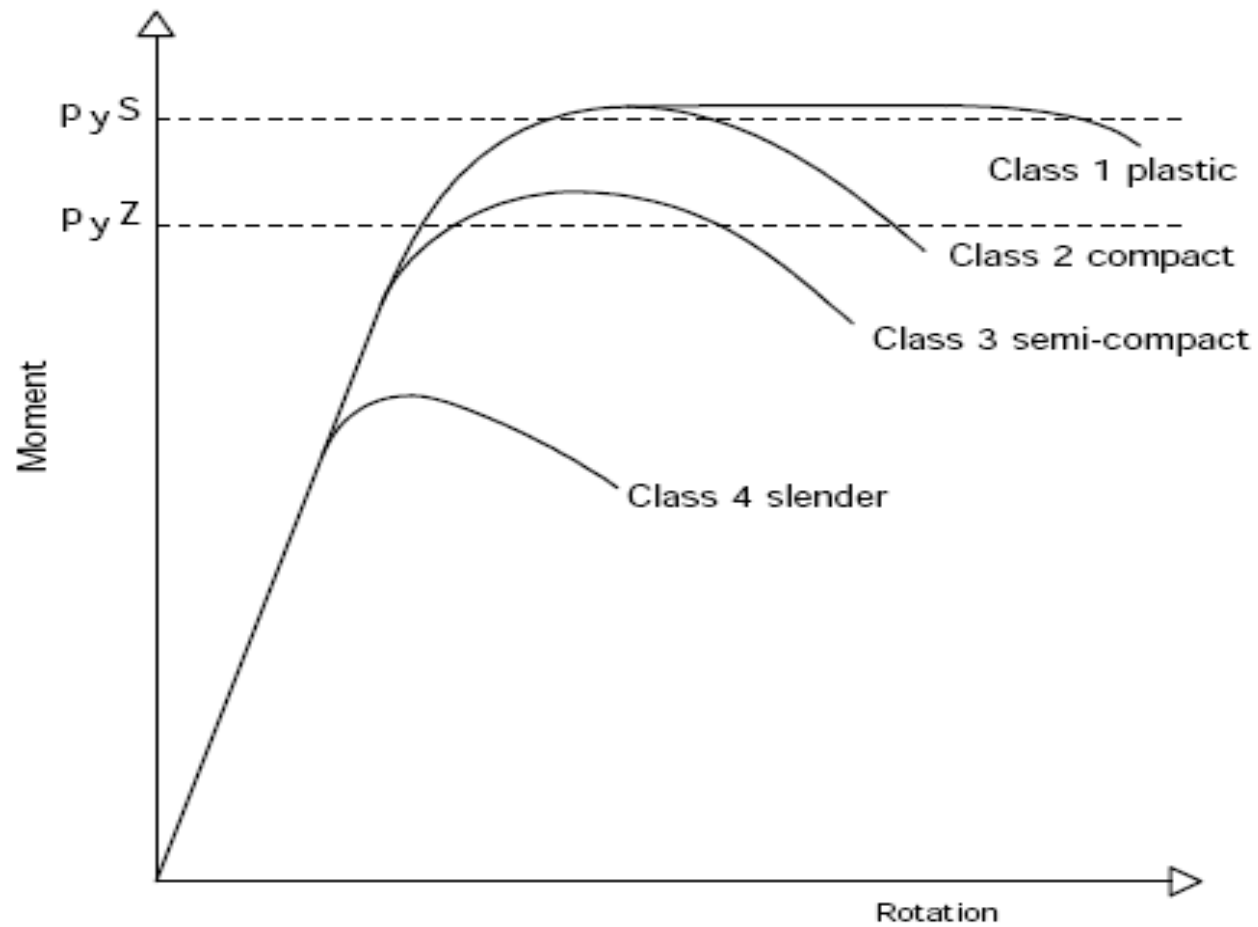
This is the basis of the  
section classification approach

## Section Class

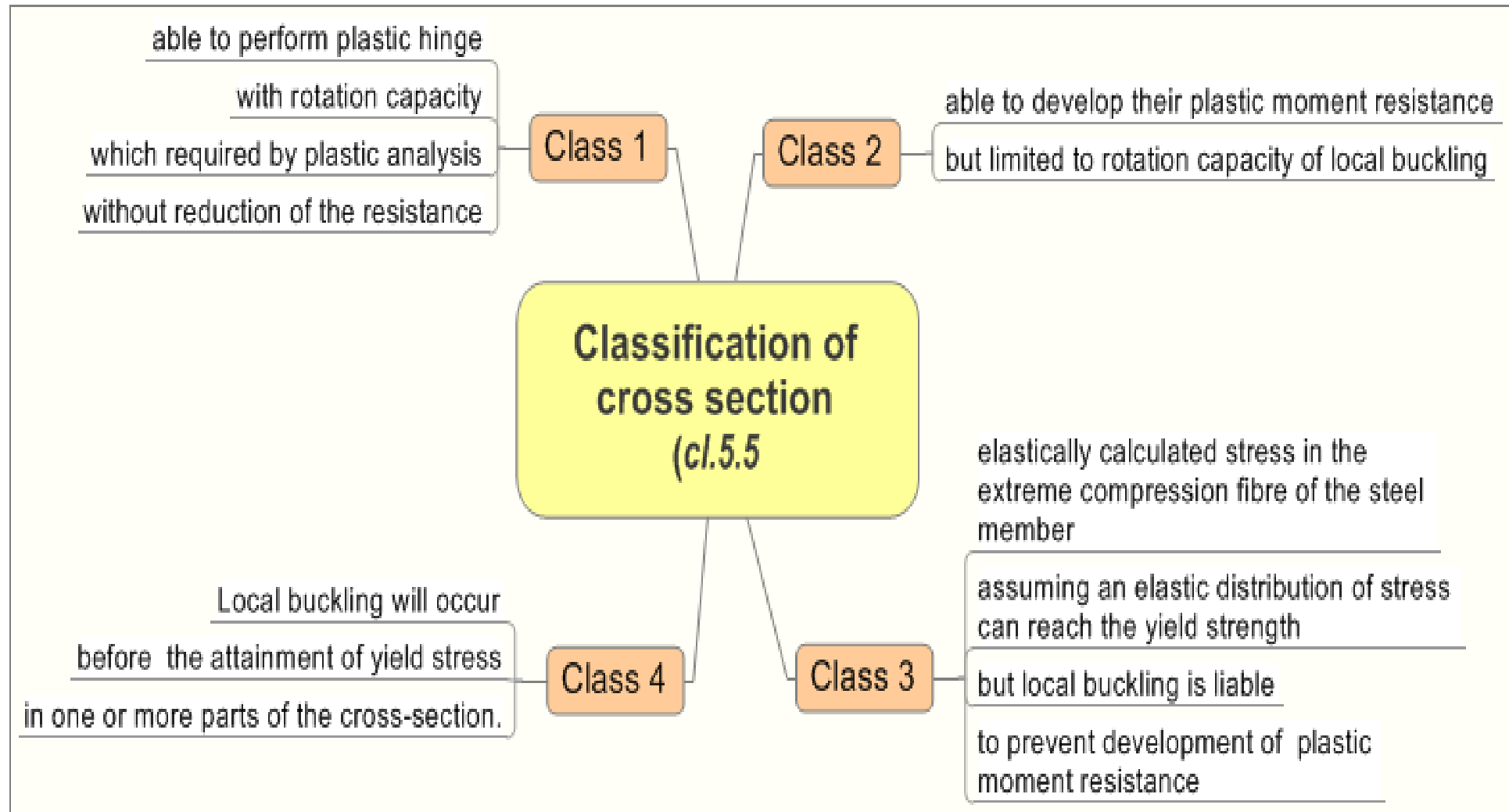
### Classification

- EC3 defines four classes of cross section.
- The class into which a particular cross section falls depends upon
  - slenderness of each element (defined by a width-to-thickness ratio)
  - the compressive stress distribution
  - Classes are defined in terms of performance requirements for resistance of bending moments

## Section Class



# Section Class





# SECTION CLASS

Classification process

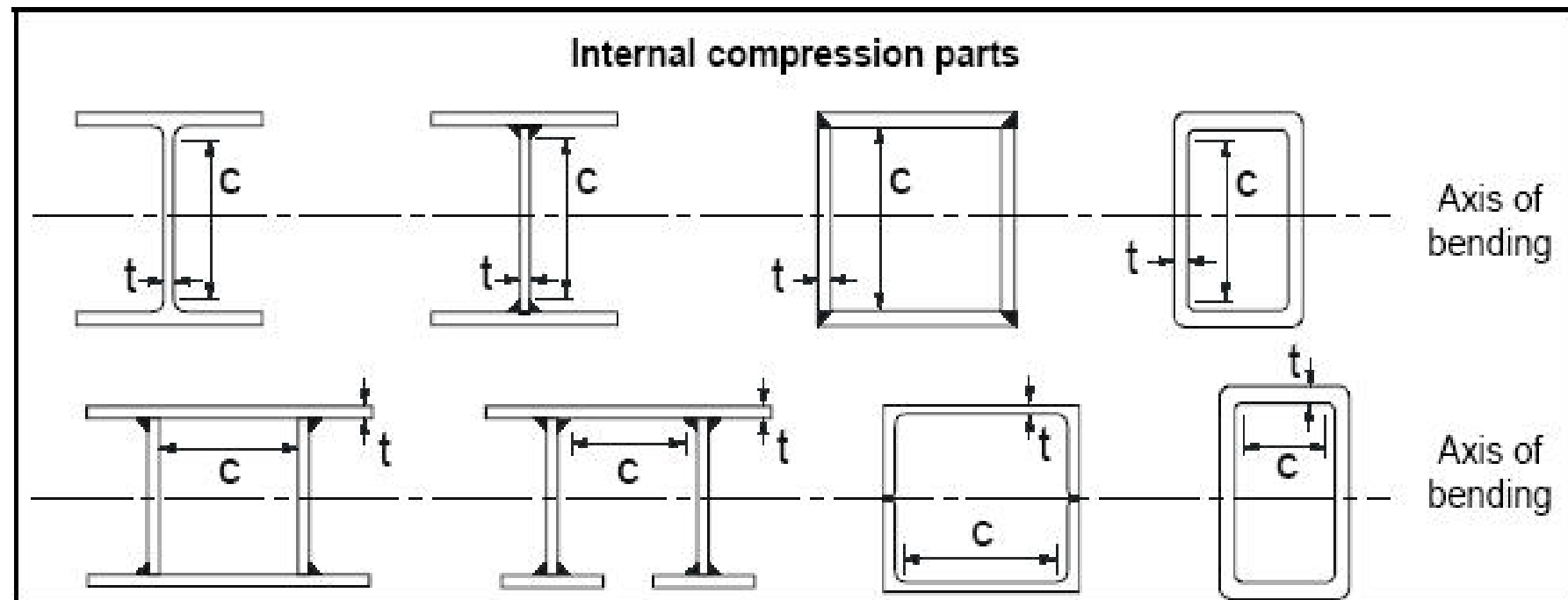
Limiting width-to-thickness ratios for sections refer Table 5.2  
EN 1993:11:2005

Classification process follows five basic steps

- Evaluate the slenderness ratio ( $c/T$  or  $d/t$ )
- Evaluate the parameter  $\varepsilon$
- Determine the class of that element based on limiting value of thickness ratio.
- Classify the complete cross-section according to the least favorable classification

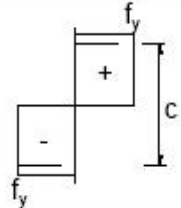
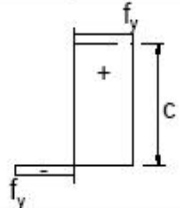
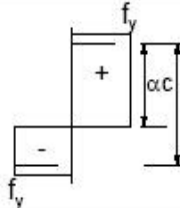
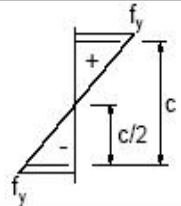
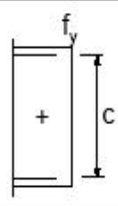
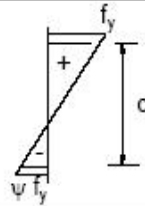
# Limiting width-to-thickness ratio

## Section Class



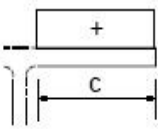
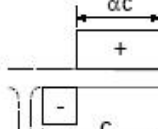
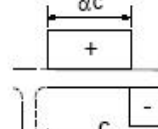
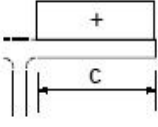
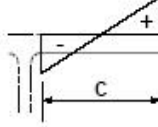
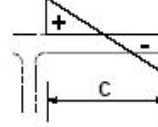
# Limiting width-to-thickness ratio

## SECTION CLASS

| Class   | Part subject to bending   | Part subject to compression  | Part subject to bending and compression  |      |      |      |
|---|---|--|--|------|------|------|
| Stress distribution in parts (compression positive) |    |    |   |      |      |      |
| 1   | $c/t \leq 72\varepsilon$  | $c/t \leq 33\varepsilon$   | when $\alpha > 0,5$ : $c/t \leq \frac{396\varepsilon}{13\alpha - 1}$<br>when $\alpha \leq 0,5$ : $c/t \leq \frac{36\varepsilon}{\alpha}$       |      |      |      |
| 2   | $c/t \leq 83\varepsilon$  | $c/t \leq 38\varepsilon$   | when $\alpha > 0,5$ : $c/t \leq \frac{456\varepsilon}{13\alpha - 1}$<br>when $\alpha \leq 0,5$ : $c/t \leq \frac{41,5\varepsilon}{\alpha}$     |      |      |      |
| Stress distribution in parts (compression positive) |  |  |   |      |      |      |
| 3   | $c/t \leq 124\varepsilon$   | $c/t \leq 42\varepsilon$   | when $\psi > -1$ : $c/t \leq \frac{42\varepsilon}{0,67 + 0,33\psi}$<br>when $\psi \leq -1^*)$ : $c/t \leq 62\varepsilon(1 - \psi)\sqrt{-\psi}$ |      |      |      |
| $\varepsilon = \sqrt{235/f_y}$                      | $f_y$   | 235  | 275  | 355  | 420  | 460  |
|   | $\varepsilon$   | 1,00   | 0,92   | 0,81 | 0,75 | 0,71 |

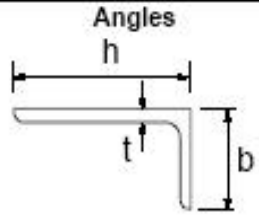
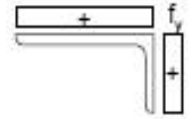
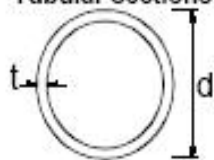
# Limiting width-to-thickness ratio

## SECTION CLASS

| Outstand flanges                                    |   |  |   |                 |      |      |
|---|---|--|---|-----------------|------|------|
|   |   | Rolled sections  |   | Welded sections |      |      |
| Class   | Part subject to compression   | Part subject to bending and compression  |   |                 |      |      |
|   |   | Tip in compression   |   | Tip in tension  |      |      |
| Stress distribution in parts (compression positive) |    |    |    |                 |      |      |
| 1   | $c/t \leq 9\epsilon$  | $c/t \leq \frac{9\epsilon}{\alpha}$  | $c/t \leq \frac{9\epsilon}{\alpha\sqrt{\alpha}}$                                      |                 |      |      |
| 2   | $c/t \leq 10\epsilon$   | $c/t \leq \frac{10\epsilon}{\alpha}$   | $c/t \leq \frac{10\epsilon}{\alpha\sqrt{\alpha}}$                                     |                 |      |      |
| Stress distribution in parts (compression positive) |  |  |  |                 |      |      |
| 3   | $c/t \leq 14\epsilon$   | $c/t \leq 21\epsilon\sqrt{k_\sigma}$<br>For $k_\sigma$ see EN 1993-1-5               |   |                 |      |      |
| $\epsilon = \sqrt{235/f_y}$                         | $f_y$   | 235  | 275   | 355             | 420  | 460  |
|   | $\epsilon$  | 1,00   | 0,92  | 0,81            | 0,75 | 0,71 |

# Limiting width-to-thickness ratio

## Section Class

|   |  |  |      |  |      |      |
|---|--|--|------|--|------|------|
| Refer also to "Outstand flanges"<br>(see sheet 2 of 3)                              |  |  |      | Does not apply to angles in continuous contact with other components |      |      |
| Class   | Section in compression   |  |      |  |      |      |
| Stress distribution across section (compression positive)                           |  |  |      |  |      |      |
| 3   | $h/t \leq 15\epsilon : \frac{b+h}{2t} \leq 11,5\epsilon$                           |  |      |  |      |      |
| <b>Tubular sections</b>   |  |  |      |  |      |      |
|  |  |  |      |  |      |      |
| Class   | Section in bending and/or compression  |  |      |  |      |      |
| 1   | $d/t \leq 50\epsilon^2$  |  |      |  |      |      |
| 2   | $d/t \leq 70\epsilon^2$  |  |      |  |      |      |
| 3   | $d/t \leq 90\epsilon^2$  |  |      |  |      |      |
| NOTE For $d/t > 90\epsilon^2$ see EN 1993-1-6.                                      |  |  |      |  |      |      |
| $\epsilon = \sqrt{235/f_y}$   | $f_y$  | 235  | 275  | 355  | 420  | 460  |
|   | $\epsilon$   | 1,00   | 0,92 | 0,81   | 0,75 | 0,71 |
|   | $\epsilon^2$   | 1,00   | 0,85 | 0,66   | 0,56 | 0,51 |

# Summary

- Structural sections may be considered as an assembly of individual plate elements.
- Plate elements may be **internal** or **outstand**
- When loaded in compression these plates may **buckle locally**
- Local buckling may **limit the load carrying capacity** of the section by **preventing the attainment of yield** strength
- Premature failure due to local buckling may be avoided by limiting the **width to thickness ratio** - or **slenderness** - of individual elements within the cross section.
- This is the basis of the **section classification** approach.
- EC3 defines four classes of cross-section.
- The class into which a particular cross-section falls depends upon the **slenderness** of each element and the compressive **stress distribution**

*Thank You*